

# The Finite Element Method: Introduction

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# Outline

- Introduction & historical developments
- Theory: variational calculus, weak form
- The finite element method
- 1D structural mechanics example
- Element types
- Advanced topics



- Basic idea of FEM: Simplify a complex geometry into simple shapes called finite elements
- Solve the simplified problem inside each finite element
- Obtain the solution to the complex problem by assembling the solution of each finite element

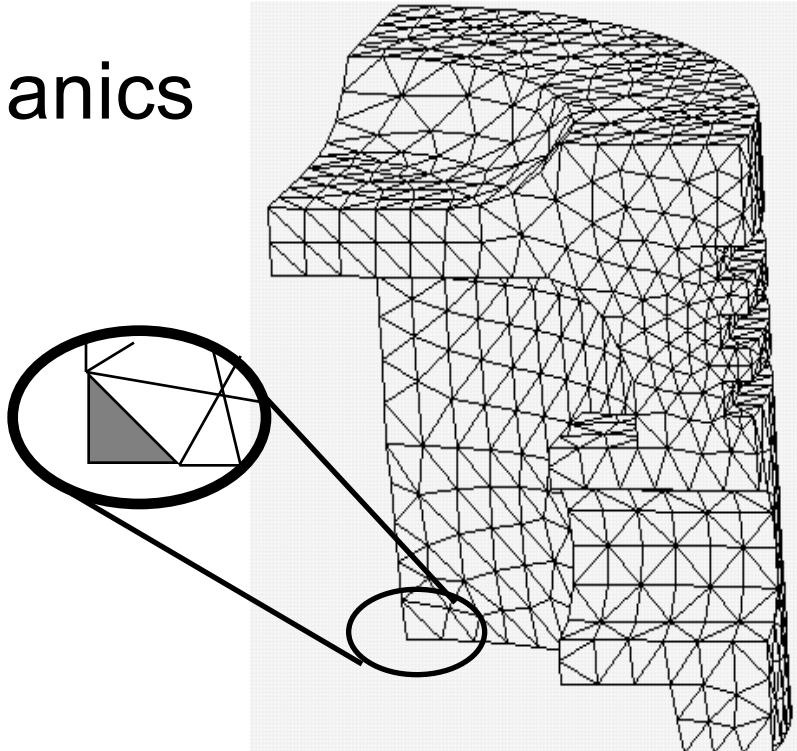
- **Example: Structural mechanics**

$[K]$  = Stiffness matrix of the part  
(Sum of all elements)

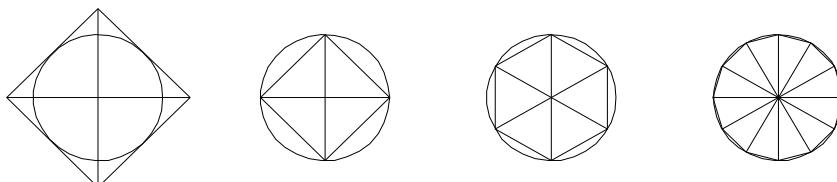
$\{U\}$  = Components of the displacements  
of the single nodes of the part

$\{F\}$  = Components of the loads of  
the single nodes of the part

$$[K] * \{U\} = \{F\}$$



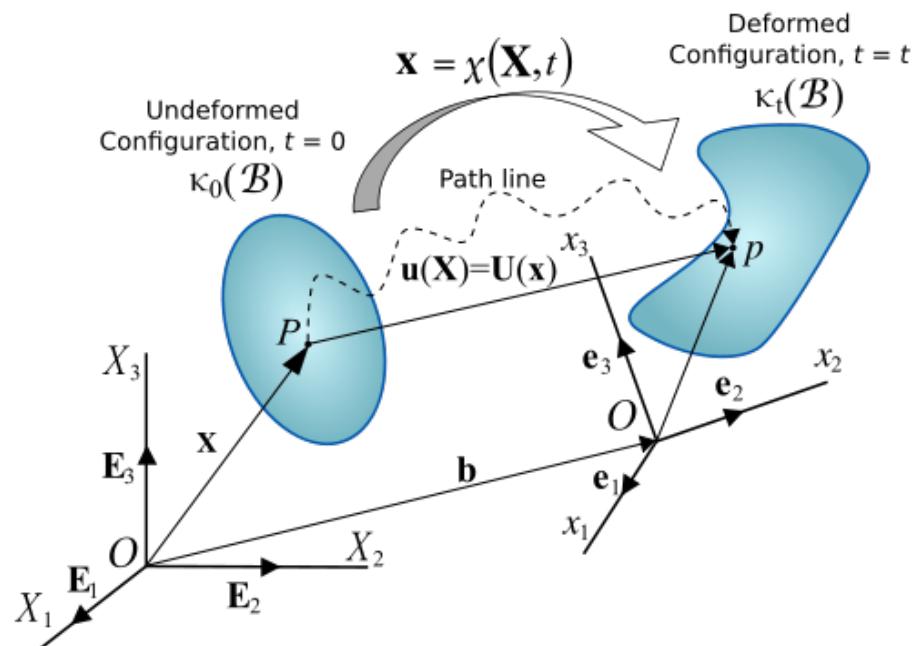
- Approach the original problem with mesh refinement



# Historical developments

- 1950s: Turner, Boeing, stiffness method
- 1960s: Zienkiewicz, Clough, FEM technology development
- 1970s: FEM software development (ANSYS, NASTRAN, ABAQUS)
- 1990s: Introduction of generalized FEMS (XFEM, isogeometric FEM)
- 2000: FEM is the standard tool for structural analysis

- Deformation map  
 $\chi: B_0 \rightarrow B$
- Deformation gradient  
 $F = \frac{\partial \chi}{\partial X}$
- Constitutive relation  
 $P = \frac{\delta W}{\delta F} = f(x, F, \dot{F}, \xi, \dots)$
- Mechanical equilibrium



$$\min_{\chi} W \Rightarrow \nabla \cdot P = f$$

# Theory: Weak form

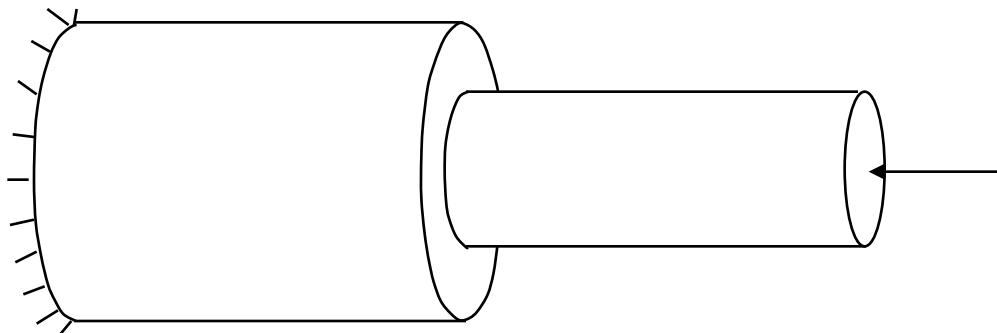
- Test functions,  $v$

$$\int v : \nabla \cdot P dV = \int v : f dV \quad \forall v \quad \leftrightarrow \quad \nabla \cdot P = f$$

- Integration by parts

$$\int \nabla v : P dV - \underbrace{\int v : P \cdot n dS}_{\text{Boundary condition}} = \int v : f dV \quad \forall v$$

- Example 1D bar



- Discrete mesh



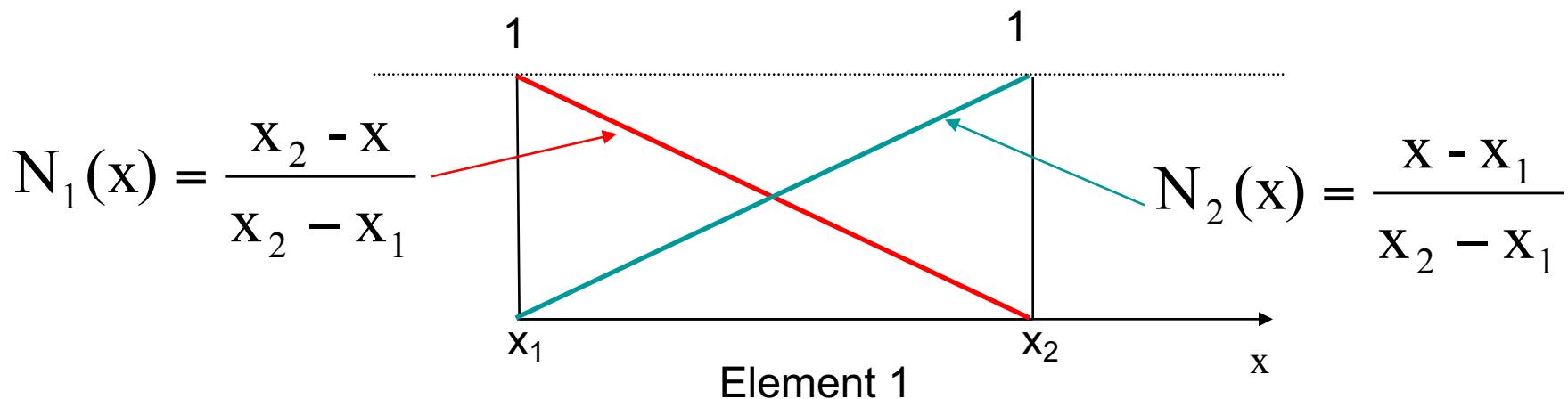
Node 1                  Node 2                  Node 3

# FEM: Shape functions (i)

- Displacement field is approximated by shape functions

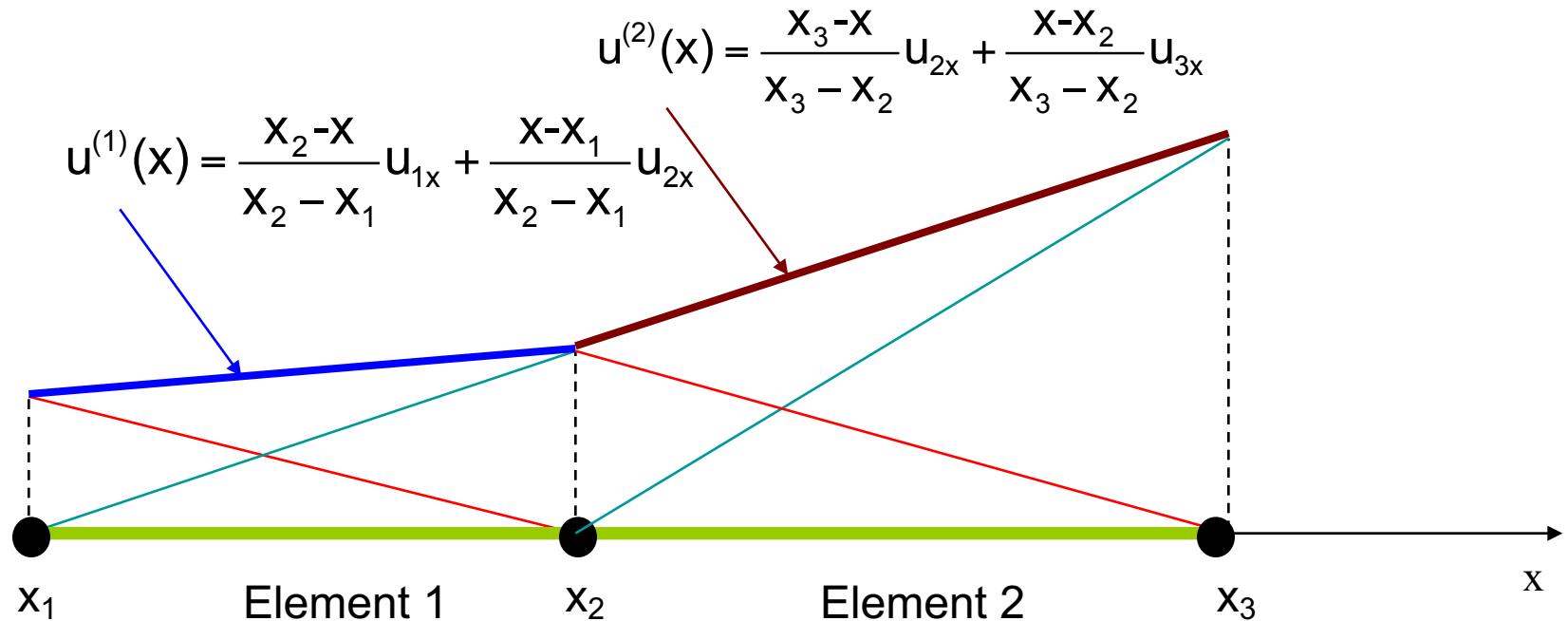
$$u(x) = \sum_i N_i(x) u_i$$

- Displacement is linearly interpolated between values at nodes



# FEM: Shape functions (ii)

- Displacement is compatible across elements



- Properties:  $N_i(x_j) = \delta_{ij}$

$$\sum_i N_i(x) = 1$$

- Strain-displacement relation

$$\varepsilon(x) = \frac{du}{dx}(x) = \sum_i \frac{dN_i}{dx}(x) u_i = \sum_i B_i u_i$$

- For linear element

$$\underline{N} = [N_1(x) \ N_2(x)] = \begin{bmatrix} \frac{x_2 - x}{x_2 - x_1} & \frac{x - x_1}{x_2 - x_1} \\ \end{bmatrix}$$

$$\underline{B} = \begin{bmatrix} -1 & 1 \\ x_2 - x_1 & x_2 - x_1 \\ \end{bmatrix} = \frac{1}{x_2 - x_1} [-1 \ 1]$$

Strain is constant in a linear element

- Galerkin: Approximate test functions and displacement using shape functions

$$u(x) = \sum_i N_i(x) u_i \quad v(x) = \sum_i N_i(x) v_i$$

- Discrete weak form

$$\nu_i^T \left( \int B_i^T E B_j dV \right) u_j = \nu_i^T \left( \int N_i^T f dV \right) \quad \forall \nu_i$$

# FEM: Stiffness matrix

- For 1D element  $dV^e = A^e dx^e$

$$\Rightarrow \sum_e \left[ \left( \int_{x_1^e}^{x_2^e} B^T EBA dx^e \right) u^e \right] = \sum_e \left[ \int_{x_1^e}^{x_2^e} N^T fA dx^e \right]$$

- Where, stiffness matrix,  $K^e$ , is given by

$$K^e = \int_{x_1^e}^{x_2^e} B^T EBA dx^e = \frac{A^e E}{L^e} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

# FEM: Global assembly

- Global stiffness matrix is assembled by combining element stiffness matrices at the connecting nodes

$$K = \sum_e K^e = \begin{bmatrix} \frac{EA^1}{L^1} & -\frac{EA^1}{L^1} & 0 \\ -\frac{EA^1}{L^1} & \frac{EA^1}{L^1} & 0 \\ 0 & 0 & 0 \end{bmatrix} + \begin{bmatrix} 0 & 0 & 0 \\ 0 & \frac{EA^2}{L^2} & -\frac{EA^2}{L^2} \\ 0 & -\frac{EA^2}{L^2} & \frac{EA^2}{L^2} \end{bmatrix}$$

Element 1      Element 2

Node 1      Node 2      Node 3

# FEM: Boundary conditions

- Essential boundary conditions: Enforced in test function space directly

$$u(0) = 0 \Rightarrow v(0) = 0$$

$$K_{BC} = \begin{bmatrix} & * & * \\ * & * & * \\ * & * & * \end{bmatrix}$$

- Natural boundary conditions: Enforced in the weak form

$$P \cdot n(0) = f_n \Rightarrow \int v : P \cdot n dS_n = \int v : f_n dS_n$$

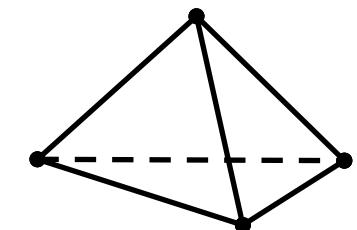
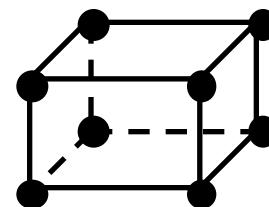
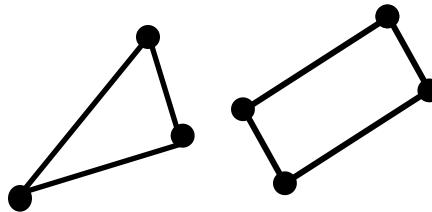
- Global system:  $K_{BC}u = \tilde{f} + \tilde{f}_n$

$$\begin{bmatrix} \frac{EA^1}{L^1} + \frac{EA^2}{L^2} & -\frac{EA^2}{L^2} \\ -\frac{EA^2}{L^2} & \frac{EA^2}{L^2} \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} 0 \\ F \end{Bmatrix}$$

- Solve for displacements

# Finite element zoo

## First order



Line  
Tetrahedral

## Second order



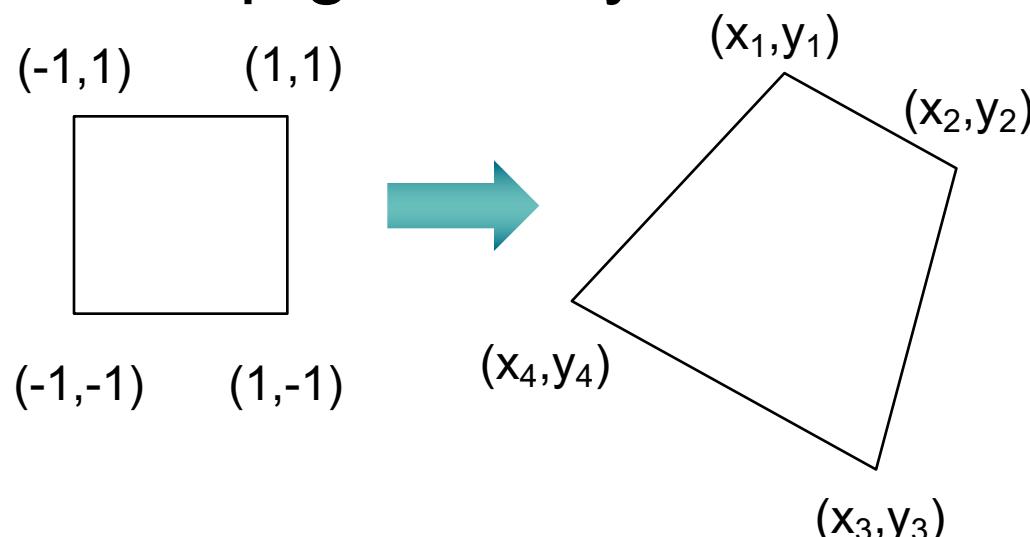
Line  
Tetrahedral

# Advanced topics: Isoparametric elements

- Define “standard element” in isoparametric domain  $\xi = [-1,1]^D$
- Use shape functions to map geometry

$$x(\xi) = \sum_i N_i(\xi) x_i$$

Same shape functions for all elements



- Stiffness matrix

$$K^e = \int_{-1}^1 (JB)^T E(JB) \det(J) d\xi$$

$$J = \frac{\partial \xi}{\partial x}$$

- Gauss quadrature

$$K^e = \int_{-1}^1 f(\xi) d\xi \approx \sum_i w_i f(\xi_i)$$

- Reduced integration: Usage of sub-optimal quadrature rule will result in zero-energy modes (eg. Hourgassing)



# Thank you for your attention!